

## **APPENDIX B**

Laboratory Testing  
Tabulated Test Results  
Individual Test Results  
Soil Chemistry Results

## LABORATORY TESTING

- A. Samples were reviewed along with field logs to determine which would be analyzed further. Those chosen for laboratory analysis were considered representative of soils that would be exposed and/or used during grading, and those deemed to be within the influence of proposed structures. Test results are presented in graphic and tabular form in this Appendix.
- B. In-situ moisture content and unit dry weight for the ring samples were determined in general accordance with ASTM D 2937.
- C. The relative strength characteristics of the soils were determined from the results of a direct shear tests on remolded and undisturbed samples. Specimens were placed in contact with water at least 24 hours before testing, and were then sheared under normal loads ranging from 0.5 to 2.0 kips per square foot in general accordance with ASTM D 3080.
- D. Settlement characteristics were developed from the results of one dimensional consolidation tests performed in general accordance with ASTM D 2435. The samples were typically loaded to 0.1.25 ksf, flooded with water, and then incrementally loaded to 0.25, 0.50, 1.0, 2.0, 4.0, and 8.0 ksf. The samples were allowed to consolidate under each load increment. Rebound was measured under reverse alternate loading. Compression was measured by dial gauges accurate to 0.0001 inch. Results of the consolidation tests in the form of percent consolidation versus log of pressure curves are presented in this Appendix.
- E. Expansion index tests were performed on bulk soil samples in accordance with ASTM D 4829. The samples were surcharged under 144 pounds per square foot at moisture content of near 50% saturation. The samples were then submerged in water for 24 hours and the amount of expansion was recorded with a dial indicator.
- F. Maximum density tests were performed to estimate the moisture-density relationship of typical soil materials. The tests were performed in accordance with ASTM 1557.
- G. The gradation characteristics of selected samples were made by hydrometer (in accordance with ASTM D 422) and sieve analysis procedures. Selected samples were soaked in water until individual soil particles were separated and then washed on the No. 200 mesh sieve, oven dried, weighed to calculate the percent passing the No. 200 sieve and then mechanically sieved. Additionally, hydrometer analyses were performed to assess the distribution of the minus No. 200 mesh material of selected samples. The hydrometer test was run using sodium hexametaphosphate as a dispersing agent.

H. Concrete and metal corrosion potential of the near surface soil was determined by measuring pH, resistivity, and soluble sulfate and soluble chloride contents. The tests were performed by Capco.

### TABULATED TEST RESULTS

#### REMOLDED SAMPLES

BORING AND DEPTH	B-1 @ 0-5'	B-3 @ 0-5'
USCS	SM	SC/CL
MAXIMUM DENSITY (pcf)	131	122.5
OPTIMUM MOISTURE (%)	8.5	9
COHESION (psf)	0	180
ANGLE OF INTERNAL FRICTION	37	24
EXPANSION INDEX	5	48
GRAIN SIZE DISTRIBUTION (%)		
GRAVEL	7.7	0.4
SAND	63.5	47.5
SILT	17.5	24.1
CLAY	11.3	28.0

#### GRAIN SIZE DISTRIBUTION (%)

BORING AND DEPTH	1 @ 25'	1 @ 30'	1 @ 40'	1 @ 45'
GRAVEL	0	99.7	0.8	1.1
SAND	44.5	0	45.5	74.9
SILT	34.7	0.3	27.9	1.1
CLAY	20.8	0	25.8	22.9

#### BORING AND DEPTH 2 @ 22'

GRAVEL	0
SAND	95.6
SILT	3.6
CLAY	0.8

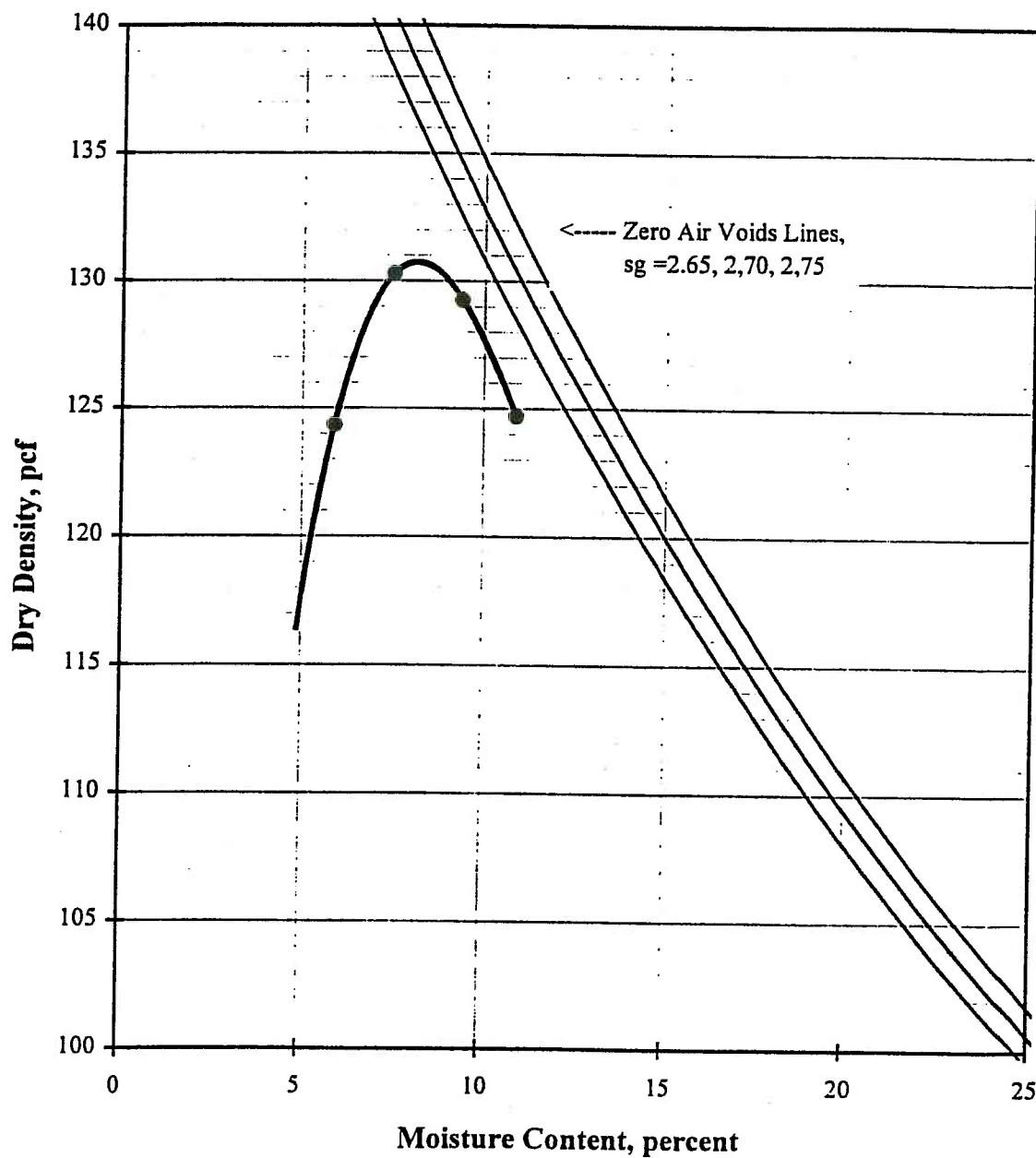
**MAXIMUM DENSITY / OPTIMUM MOISTURE**

ASTM D 1557-91 (Modified)

Job Name: Santa Barbara Zoological Gardens  
 Sample ID: B 1 @ 0 - 5  
 Location:  
 Description: Silty Sand

Procedure Used: A  
 Prep. Method: Moist  
 Rammer Type: Manual

		Sieve Size	% Retained
<b>Maximum Density:</b>	<b>131 pcf</b>	3/4"	0.0
<b>Optimum Moisture:</b>	<b>8.5%</b>	3/8"	0.0
Corrected for Oversize (ASTM D4718)		#4	11.0



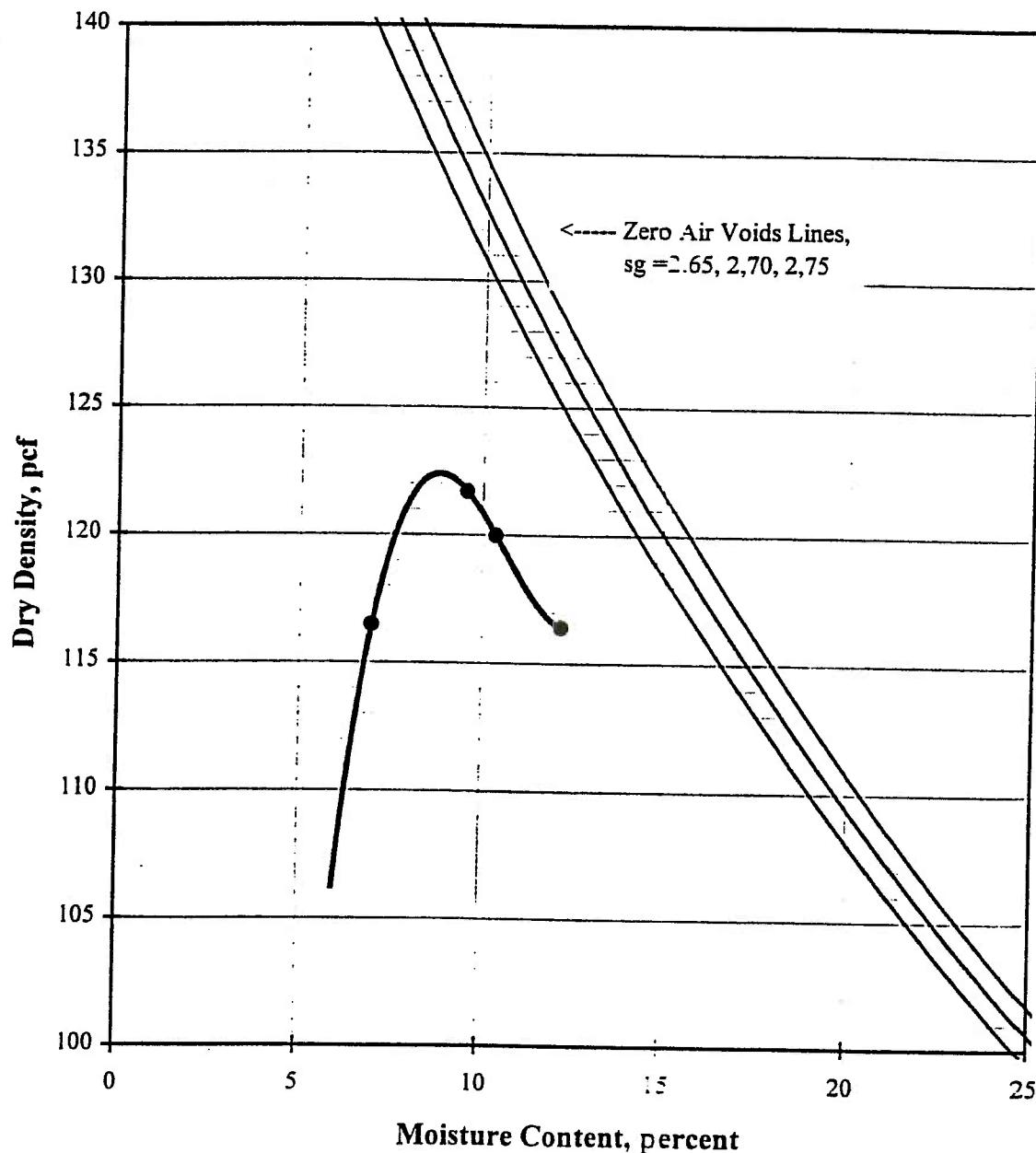
**MAXIMUM DENSITY / OPTIMUM MOISTURE**

ASTM D 1557-91 (Modified)

Job Name: Santa Barbara Zoological Gardens  
 Sample ID: B 3 @ 0 - 5  
 Location:  
 Description: Silty Clayey Sand

Procedure Used: A  
 Prep. Method: Moist  
 Rammer Type: Manual

		Sieve Size	% Retained
Maximum Density:	122.5 pcf	3/4"	0.0
Optimum Moisture:	9%	3/8"	0.0
		=4	0.0

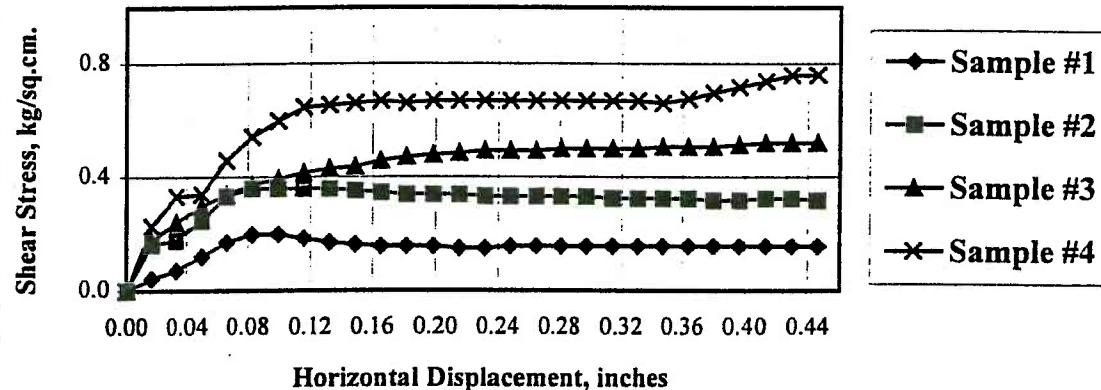
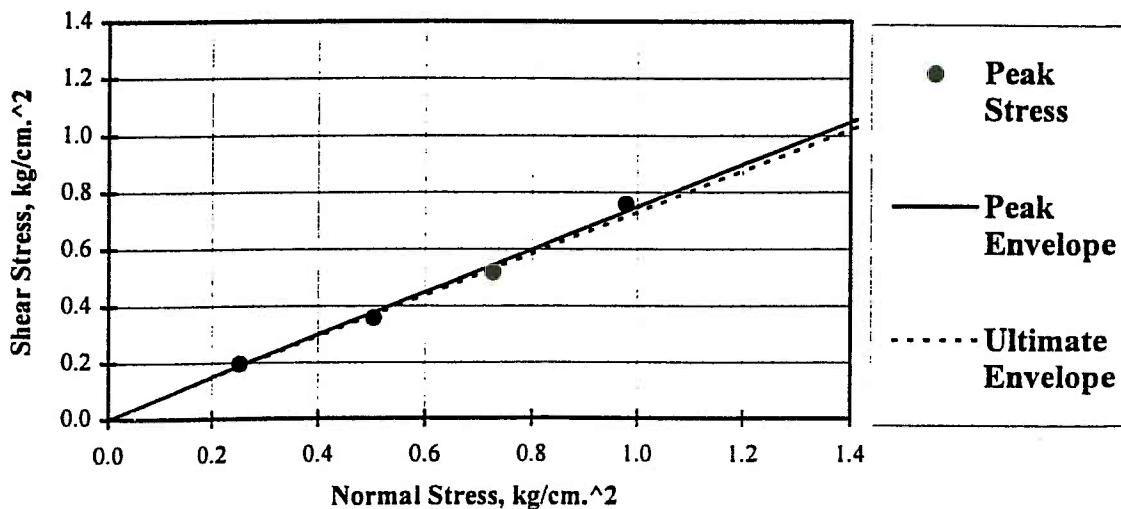


**DIRECT SHEAR** 080-90 (modified for unconsolidated, undrained conditions)

Santa Barbara Zoological Gardens  
 B 1 @ 0 - 5  
 Silty Sand  
 Remolded

Initial Dry Density: 113.9 pcf  
 Initial Moisture Content: 8.5 %  
 Peak Friction Angle ( $\phi$ ): 37°  
 Cohesion (c): 0.000 kg/cm<sup>2</sup> (0 psf)

Sample No.	1	2	3	4	Average
Initial					
Dry Density, pcf	113.8	113.9	113.9	113.9	113.9
Moisture Content, %	8.5	8.5	8.5	8.5	8.5
Saturation, %	49	49	49	49	49
At Test					
Moisture Content, %	17.0	16.1	16.8	16.5	16.6
Saturation, %	98	93	97	95	96
Normal Stress, kg/cm <sup>2</sup>	0.25	0.51	0.73	0.98	
Peak Stress, kg/cm <sup>2</sup>	0.20	0.36	0.52	0.76	
Ultimate Stress, kg/cm <sup>2</sup>	0.15	0.32	0.52	0.76	

**SHEAR vs. NORMAL STRESS DIAGRAM**

**DIRECT SHEAR**

080-90 (modified for unconsolidated, undrained conditions)

Santa Barbara Zoological Gardens

B 3 @ 0 - 5

Silty Clayey Sand

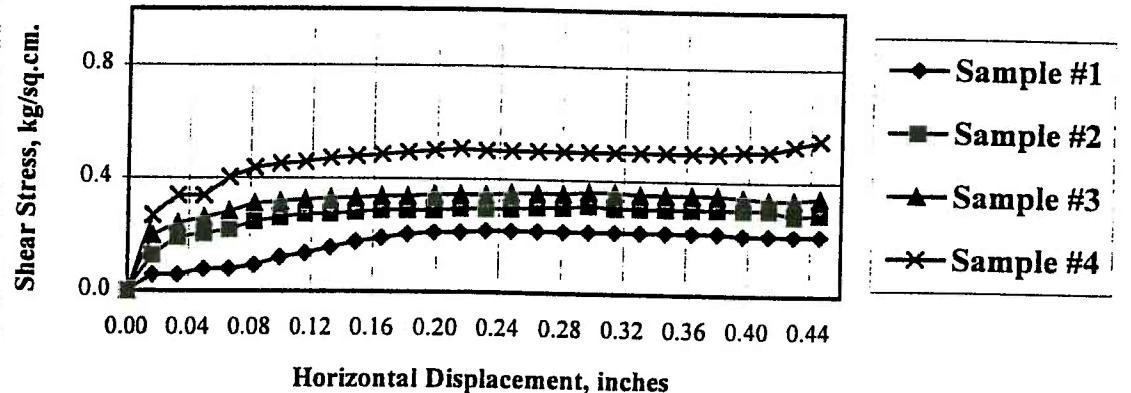
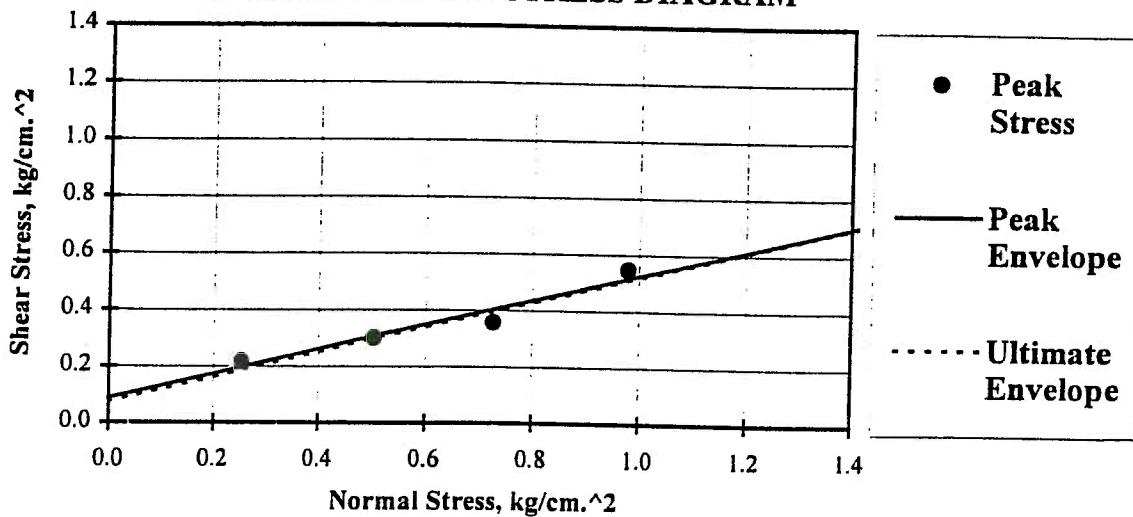
Remolded

Initial Dry Density: 109.4 pcf

Initial Moisture Content: 9.5 %

Peak Friction Angle ( $\phi$ ): 24°Cohesion (c): 0.087 kg/cm<sup>2</sup> (180 psf)

Sample No.	1	2	3	4	Average
Initial					
Dry Density, pcf	109.4	109.4	109.4	109.4	109.4
Moisture Content, %	9.5	9.5	9.5	9.5	9.5
Saturation, %	49	49	49	49	49
At Test					
Moisture Content, %	19.4	18.4	19.1	19.1	19.0
Saturation, %	99	94	98	98	97
Normal Stress, kg/cm <sup>2</sup>	0.25	0.51	0.73	0.98	
Peak Stress, kg/cm <sup>2</sup>	0.22	0.30	0.36	0.55	
Ultimate Stress, kg/cm <sup>2</sup>	0.21	0.29	0.34	0.55	

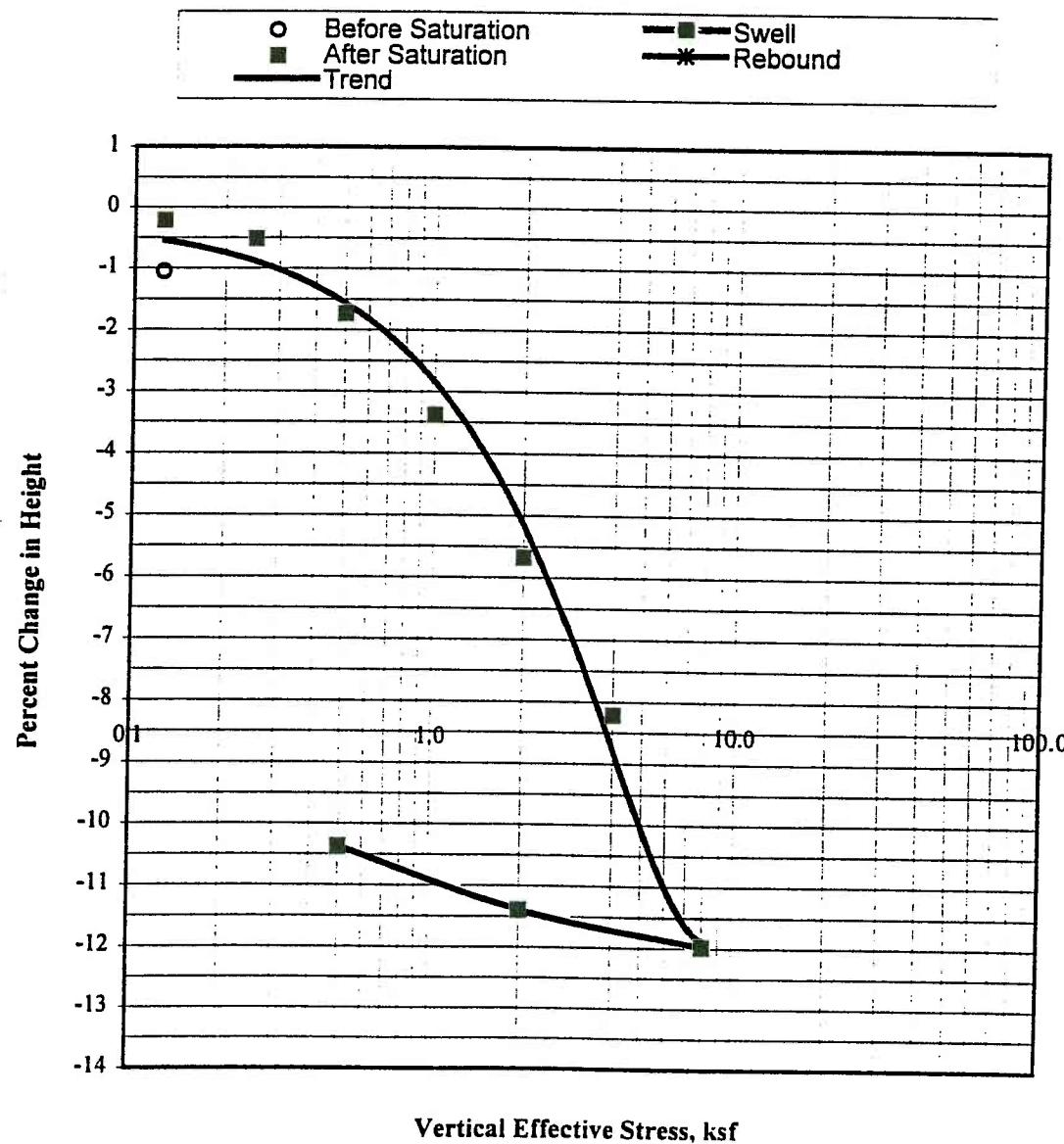
**SHEAR vs. NORMAL STRESS DIAGRAM**

**CONSOLIDATION TEST**

ASTM D 2435-90

Santa Barbara Zoological Gardens  
1 @ 10  
Silty Clay  
Ring Sample

Initial Dry Density: 112.3 pcf  
Initial Moisture, %: 17.7%  
Specific Gravity: 2.67 (assumed)  
Initial Void Ratio: 0.484

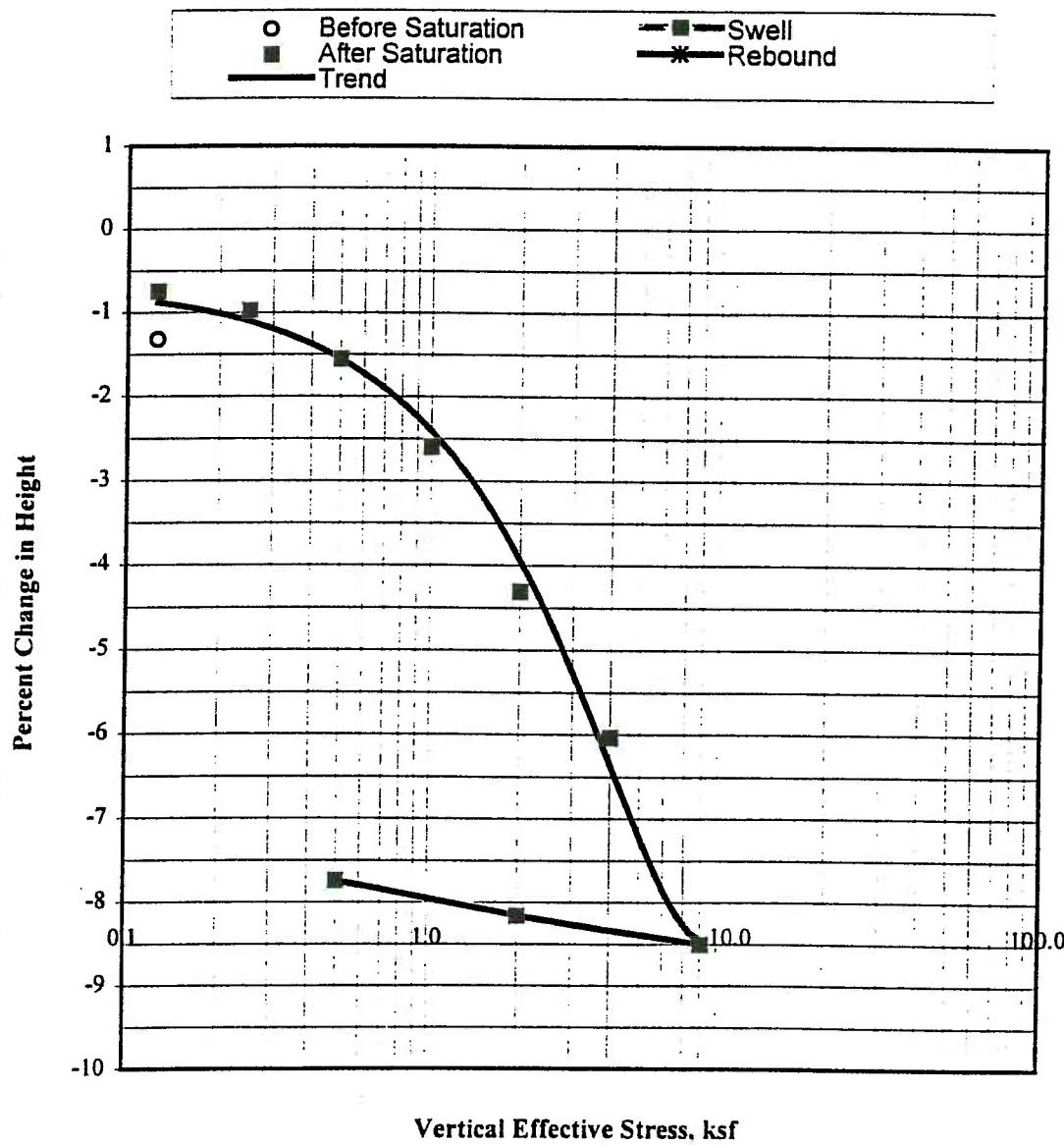
**% Change in Height vs Normal Pressure Diagram**

**CONSOLIDATION TEST**

ASTM D 2435-90

Santa Barbara Zoological Gardens  
5 @ 2  
Silty Sand  
Ring Sample

Initial Dry Density: 109.0 pcf  
Initial Moisture, %: 11.9%  
Specific Gravity: 2.67 (assumed)  
Initial Void Ratio: 0.529

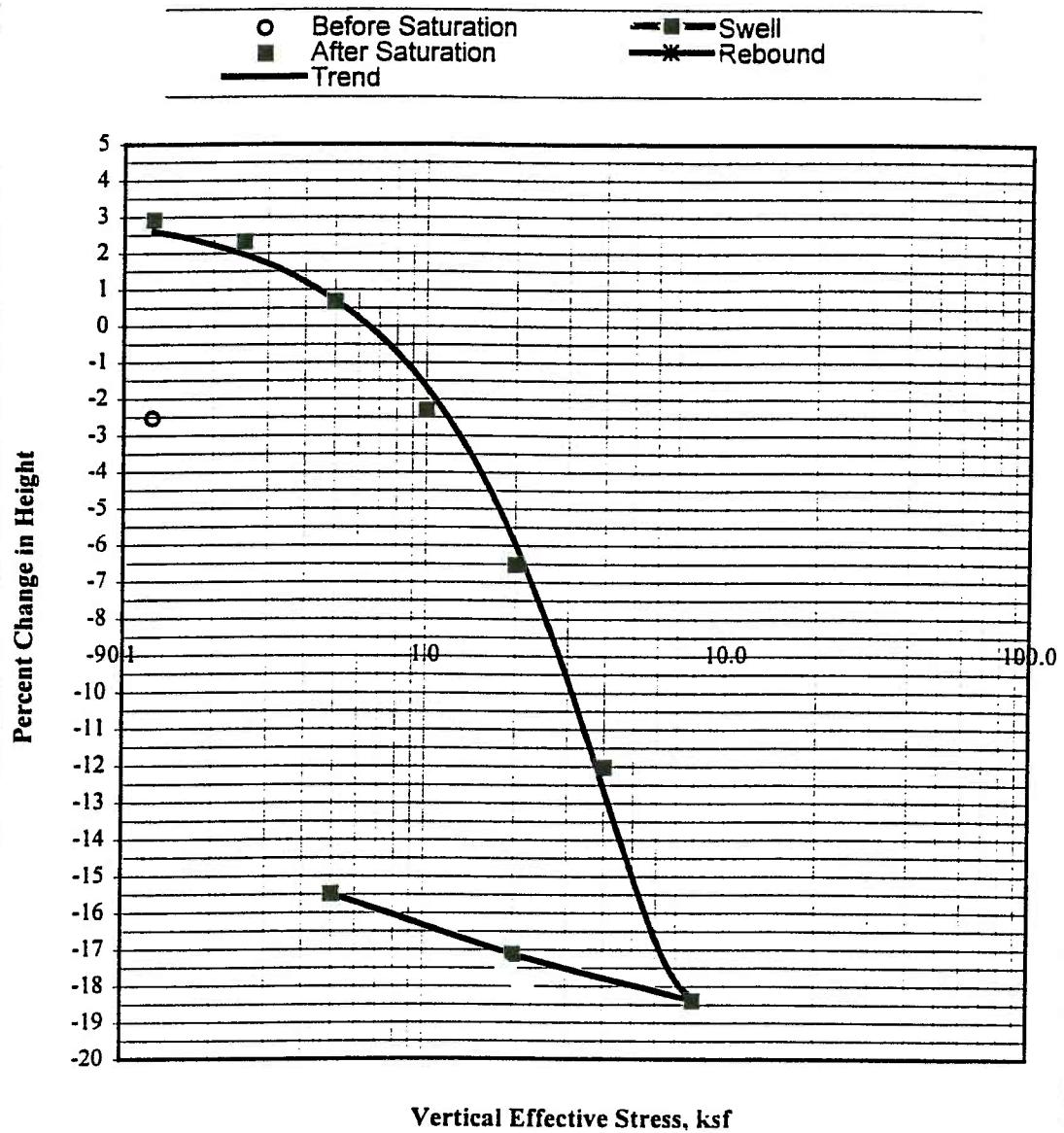
**% Change in Height vs Normal Pressure Diagram**

**CONSOLIDATION TEST**

ASTM D 2435-90

Santa Barbara Zoological Gardens  
2 @ 10  
Silty Clay  
Ring Sample

Initial Dry Density: 114.2 pcf  
Initial Moisture, %: 15.2%  
Specific Gravity: 2.67 (assumed)  
Initial Void Ratio: 0.459

**% Change in Height vs Normal Pressure Diagram**

Capco Analytical Services INC. (CAS)  
1536 Eastman Avenue, Suite B  
Ventura CA 93003  
(805) 644-1095

Client: Earth Systems  
Sample ID: B1 @ 0-5  
Date Received: 06/27/01  
Date Sampled: N/A

Sample Matrix: Soil  
CAS LAB NO: 01128901

WET CHEMISTRY ANALYSIS SUMMARY

COMPOUND	RESULT	UNITS	DF	PQL	METHOD	ANALYZED
*Chloride	22	mg/Kg	1	10	300.0M	06/27/01
pH	8.2	S.U.	1	---	9045	06/27/01
*Resistivity	1880	ohms-cm	1	3	CA test 424	06/27/01
*Sulfate	610	mg/Kg	5	50	300.0M	06/27/01

\*Sample was analyzed on a 1:10 soil/water extract. Results were reported based on the original soil sample weight.

PQL: Practical Quantitation Limit

BQL: Below Practical Quantitation Limit

  
Principal Analyst

Capco Analytical Services INC. (CAS)  
1536 Eastman Avenue, Suite B  
Ventura CA 93003  
(805) 644-1095

Client: Earth Systems  
Sample ID: B3 @ 0-5  
Date Received: 06/27/01  
Date Sampled: N/A

Sample Matrix: Soil  
CAS LAB NO: 01128902

WET CHEMISTRY ANALYSIS SUMMARY

COMPOUND	RESULT	UNITS	DF	PQL	METHOD	ANALYZED
*Chloride	29	mg/Kg	1	10	300.0M	06/27/01
pH	7.4	S.U.	1	---	9045	06/27/01
*Resistivity	4610	ohms-cm	1	3	CA test 424	06/27/01
*Sulfate	79	mg/Kg	1	10	300.0M	06/27/01

\*Sample was analyzed on a 1:10 soil/water extract. Results were reported based on the original soil sample weight.

PQL: Practical Quantitation Limit

BQL: Below Practical Quantitation Limit

  
Principal Analyst

## **APPENDIX C**

### **Liquefaction Analyses**









**CPT-LIQUEFY.XLS - A SPREADSHEET FOR EMPIRICAL ESTIMATION OF LIQUEFACTION POTENTIAL USING CPT DATA**  
 Developed 2001 by Shelton L. Stringer, GE, Earth Systems Southwest

Project: Santa Barbara Zoo

Job No: VT-22803-01

Date: 7/11/01

Sounding: CPT-1

Plot: 1

Methods: Liquefaction Analysis using 1996 & 1998 NCEER workshop methods (Youd & Idriss)

Post-liquefaction Settlement Analysis from Tokimatsu & Seed (1987)

Dry Sand Settlement by Pradel, ASCE Journal of G&GE, Vol 124, No. 4

**EARTHQUAKE INFORMATION:**

Magnitude: 7 7.5

PGA, g: 0.70 0.59

MSF: 1.19 1.30

GWT, feet: 13.0

$$\text{Induced CSR (M=7.5)} = 0.65 \cdot \text{PGA} \cdot (\rho_o/\rho') \cdot \text{rd}/\text{MSF}$$

$$\text{Clean Sand } Q_{c1n} = C_o \cdot K_c \cdot K_u \cdot Q_c$$

Use Tokimatsu & Seed (0) or Ishihara & Yoshimine (1): 0

Required SF: 1.25

Min SF of Liquefiable Layers: 0.39

Avg SF of Liquefiable Layers: 0.57

Total Liquefied Thickness (feet)	7.2
Total Induced Subsidence (inches)	2.0
Volumetric Strain Factor (%)	0.52

Layer Depth (feet)	Tip Qc (tsf)	Friction Fs	Friction Ratio %	Total Unit Wt. (pcf)	Eff. Stress at Midpt. p'o (tsf)	rd	Corrected			Rel. Dens. (%)	Est. Pines (%)	H (m)	Clean Sand			M=7.5 CSR	Induced Safety Factor	Liquefac. (%)	Volumetric Strain Factor (%)			
							F	n	C <sub>o</sub>	Q <sub>c1n</sub>	I <sub>c</sub>	Dr (%)	K <sub>c</sub>	K <sub>H</sub>	Q <sub>c1n</sub>	K <sub>c</sub>	CRR <sub>s</sub>					
49.38	81.71	1.90	2.32	131	2.087	0.760	2.39	0.73	0.61	45.76	2.41	44	60	2.37	0.25	1.46	158.5	0.87	0.450	0.447	0.88	0.52
49.54	74.27	2.04	2.75	131	2.093	0.758	2.83	0.76	0.60	40.66	2.50	39	65	2.78	0.25	1.46	165.0	0.87	Infin.	0.446	Non-Liq.	0.00
49.70	65.29	1.86	2.85	131	2.098	0.757	2.95	0.78	0.59	35.10	2.56	33	75	3.10	0.25	1.46	158.8	0.87	0.452	0.446	0.88	0.50
49.87	66.79	1.45	2.18	131	2.104	0.755	2.25	0.75	0.60	36.52	2.47	35	65	2.63	0.25	1.46	140.3	0.87	0.337	0.445	0.66	0.99
50.03	59.10	1.58	2.72	131	2.110	0.753	2.82	0.79	0.58	30.77	2.59	28	75	3.28	0.25	1.46	147.4	0.87	0.378	0.444	0.74	0.80









CPT-LIQUEFY.XLS - A SPREADSHEET FOR EMPIRICAL ESTIMATION OF LIQUEFACTION POTENTIAL USING CPT DATA  
Developed 2001 by Shelton L. Stringer, GE, Earth Systems Southwest

Project: Santa Barbara Zoo  
Job No: VT-22503-01  
Date: 7/11/01  
Sounding: CPT-2

**Methods:** Liquefaction Analysis using 1996 & 1998 NCEER workshop methods (Yousif & Idriss)  
Post-liquefaction Settlement Analysis from Tokimatsu & Seed (1987)  
Dry Sand Settlement by Pradel, ASCE Journal of G&GE, Vol 124, No. 4

## **EARTHQUAKE INFORMATION:**

Magnitude: 7 7.5  
 PGA, g: 0.70 0.55  
 MSF: 1.19 130  
 GWT, feet: 15.0

### Plot: 2

$$\text{Induced CSR (M=7.5)} = 0.65 \cdot \text{PGA} \cdot (\rho_0/\rho') \cdot r_d / \text{MSF}$$

Use Tokimatsu & Seed (0) or Ishihara & Yoshimine (1): 0

**Required SF: 1.25**

Total  
Liquefied  
Thickness  
(feet)  
0.0

Total  
Induced  
Subsidence  
(inches)  
0.1